

25 May 2020

Malgy Coman
 Senior Land Use Planner
 Transport for NSW
 Sent via email

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Re: Georges Cove Marina at 146 Newbridge Road, Moorebank - Response to TfNSW matters

Dear Malgy,

This letter responds to the traffic related matters that have been raised by Transport for NSW (TfNSW) in their letter dated 30 March 2020 for the Georges Cove Marina (TfNSW Ref: SYD18/01728/04, Council Ref: DA-611/2018).

1.1 Newbridge Road/Government Macquarie Drive/Brickmakers Drive intersection

TfNSW Comment

The SIDRA modelling results indicate that the performance and capacity of the Newbridge Road / Government Macquarie Drive / Brickmakers Drive intersection will be significantly impacted by the traffic generated by the proposed development.

Measures should be proposed to mitigate these traffic impacts on the Newbridge Road / Government Macquarie Drive / Brickmakers Drive intersection. In particular, for all movements out of Brickmakers Drive, the right turn movements from Newbridge Road into Brickmakers Drive and through movements from Governor Macquarie Drive into Brickmakers Drive.

EMM Response

EMM has undertaken cumulative traffic impact assessment for all five developments within this precinct. A breakdown of traffic generation is provided in Table 1.

Table 1 Traffic generation from Georges Cove Marina developments (source: EMM letter response to RMS matters dated 20 March 2020)

	AM peak hour		PM peak hour	
	Traffic generation	Percentage	Traffic generation	Percentage
Moorebank Cove Residential	152	20.7%	152	18.2%
Moorebank Recycling Facility	43	5.9%	29	3.5%
Georges Cove Marina Commercial	61	8.3%	86	10.3%

Table 1 Traffic generation from Georges Cove Marina developments (source: EMM letter response to RMS matters dated 20 March 2020)

	AM peak hour		PM peak hour	
Georges Cove Marina Residential	181	24.7%	181	21.7%
Benedict B6 Corridor Mixed-use Development	296	40.4%	386	46.3%
Total	733	100%	834	100%

The data in the above table shows that the subject development (Georges Cove Marina Commercial) will generate 61 and 86 vehicles during the AM and PM peak hours which equates to about 8 to 10% of total development traffic. Of this development traffic, 55 and 78 vehicles will travel to/from the north during the AM and PM peak hours respectively.

Traffic volume data for the Newbridge Road/Government Macquarie Drive/Brickmakers Drive intersection show that the intersection will carry 5,535 vehicles during the AM peak hour and 6,013 vehicles during the PM peak hour respectively. The additional Georges Cove Marina commercial development traffic will account for between 1.0% to 1.3% of additional traffic at this intersection during the peak hours as shown in Table 2.

Table 2 Development traffic volume percentage during peak hours

Peak hour	Georges Cove Marina Commercial development traffic (to/from the north)	Newbridge Road/Governor Macquarie Drive/Brickmakers Drive intersection	Percentage
AM	55	5,535	1.0%
PM	78	6,013	1.3%

A detailed analysis of the SIDRA modelling results (refer to Appendix B and Appendix C) shows that some approaches of the this intersection currently operates at Level of Service (LOS) F, with significant delays. On no occasion, is this approach performing any better than LOS E during peak hours. For example, during both the AM and PM peak hours, the right turn movement from Brickmakers Drive is currently operating at LOS F. For the north approach of this intersection (Governor Macquarie Drive), the through movements are already operating at LOS E during the AM peak and LOS F during the PM peak hour. Similarly, on the eastbound approach of Newbridge Road, the right turn movement is already LOS F during the AM peak and LOS E during the PM peak hour. During both peak hours, the queuing of the right turn lanes is encroaching into the through movement. The intersection is already under considerable strain with over 5,000 vehicles during the peak hours.

The additional 55 vehicles during the AM peak and 78 vehicles during the PM peak travelling to/from the north due to the marina commercial development, equating 1.3% of total traffic, are unlikely to have any significant impact on the operation of this signalised intersection. Therefore, we believe it is unreasonable to require any mitigation measures for such a small increase of traffic at this intersection.

1.2 Brickmakers Drive and Link Road intersection - signalisation

TfNSW Comment

EMM predicted that by the time the development is constructed and operational around the year 2027-2028, the growth of the background traffic will likely meet the signal warrant for the Brickmakers Drive and Link Road intersection.

Data and information to support this statement should be provided. SIDRA modelling should be undertaken for the traffic signals at the Brickmakers Drive/Link Road intersection with updated traffic volumes. The SIDRA modelling results, proposed traffic signal phasing plan, and concept civil plan should be provided to TfNSW for review.

EMM Response

In accordance with the *Traffic Signal Design Guidelines*, the signal warrant requires the major road to have a flow exceeding 600 vehicles/hour in both directions, or a high speed limit/significant crash history and the minor road to have a flow exceeding 200 vehicles/hour in one direction.

By the time the development is constructed and operational, about 2026, there will be a 12% growth of background traffic (assuming 2% linear growth per annum). Hence, the signal warrant for Brickmakers Drive/Link Road intersection has been assessed for 2026.

EMM has undertaken traffic counts for Brickmakers Drive for 7 am–9 am and 4 pm–6 pm where peak hour traffic was determined to be 7 am–8 am and 4.30 pm–5.30 pm.

Council's Development Control Plan (DCP) stipulates using a one-hour peak-hour (ie one hour for the AM and one hour for the PM) traffic generation for developments, whereas the signal warrant assessment requires traffic movements for four peak hours of the day. For the purpose of this assessment, 7 am–9 am and 4 pm–6 pm are adopted as the four peak hours. It is assumed that the one-hour peak-hour traffic makes up 55% of the two-hour peak traffic in the respective AM and PM period. The traffic generation in the two-hour peak-periods would therefore be as presented in Table 3. As one of the actual hourly peaks (4.30 to 5.30 pm) straddles an hourly intervals, the analysis has to be split into half hourly periods to estimate the equivalent hourly volumes either side of the actual peak hour. This breakup only needed to be done for the pm peak period, however, to keep the table consistent it was done for both the am and pm peak periods. .

Table 3 Peak hour development traffic summary

AM and PM two-hour peak periods	Proportion of traffic generation over a two-hour (peak) period	Traffic generation half hourly totals (highlighted periods)		
		Brickmakers Drive northbound (318 AM / 316 PM)	Brickmakers Drive southbound (219 AM / 306 PM)	Link Road westbound (450 AM / 407 PM)
7 to 7.30 am	27.5%	159	109.5	225
7.30 to 8 am	27.5%	159	109.5	225
8 to 8.30 am	22.5%	130	89.6	184
8.30 to 9 am	22.5%	130	89.6	184
4 to 4.30 pm	22.5%	129.3	125.2	166.5
4.30 to 5 pm	27.5%	158	153	203.5
5 to 5.30 pm	27.5%	158	153	203.5
5.30 to 6 pm	22.5%	129.3	125.2	166.5

Based on this assumption, the forecast traffic volume in Brickmakers Drive and Link Road is estimated using:

Traffic volume (in 2026) = Traffic volume (2020 tube count) x traffic growth rate (1.12) + peak hour development traffic from the two half hourly totals in Table 3.

- Brickmakers Drive northbound traffic:
 - 7 am to 8 am: $656 \times 1.12 + (159 + 159) = 1,053$ vehicles

- 8 am to 9 am: $753 \times 1.12 + (130 + 130) = 1,103$ vehicles
- 4 pm to 5 pm: $321 \times 1.12 + (129.3 + 158) = 647$ vehicles
- 5 pm to 6 pm: $290 \times 1.12 + (158 + 129.3) = 612$ vehicles
- Brickmakers Drive southbound traffic:
 - 7 am to 8 am: $340 \times 1.12 + (109.5 + 109.5) = 600$ vehicles
 - 8 am to 9 am: $380 \times 1.12 + (89.6 + 89.6) = 605$ vehicles
 - 4 pm to 5 pm: $882 \times 1.12 + (125.2 + 153) = 1,266$ vehicles
 - 5 pm to 6 pm: $916 \times 1.12 + (153 + 125.2) = 1,304$ vehicles
- Link Road westbound traffic:
 - 7 am to 8 am: $(225 + 225) = 450$ vehicles (peak hour development traffic)
 - 8 am to 9 am: $(184 + 184) = 368$ vehicles
 - 4 pm to 5 pm: $(166.5 + 203.5) = 370$ vehicles
 - 5 pm to 6pm: $(203.5 + 166.5) = 370$ vehicles

Traffic flows on the major road, Brickmakers Drive, will reach or exceed 600 vehicles/hour (each way) for each of the four traffic peak hours. Traffic flows on the minor road, Link Road, will also exceed 200 vehicles/hour (one way) for each of the four traffic peak hours. These flows meet the traffic signal warrant as stipulated in Section 2.1(a) of the Traffic Signal Design Guidelines, refer Appendix D.

Hence based on the background traffic growth and the forecast development traffic, the signalisation of the intersection will be required from about 2026.

SIDRA modelling analyses the capacity of any particular intersection and does not provide any information on traffic signal warrant. Therefore, SIDRA analysis has not been undertaken for the signal warrant assessment.

Proposed traffic signal phasing plan and concept civil plan are attached in Appendix F.

1.3 Brickmakers Drive and Link Road intersection - lanes

TfNSW Comment

The signalized intersection layout shown on Figure 11 provides two right turn lanes on the Link Road, but only one through lane on the northern leg on Brickmakers Drive. It is not possible for vehicles to merge within the signalized intersection and this layout needs to be amended accordingly. Also it is not clear why there is a 60m long acceleration/merge lane on the southern leg for the single through lane on the northern leg on Brickmakers Drive.

EMM Response

The Liverpool traffic committee has approved the line marking plan for the Brickmakers Drive/Link Road intersection as a priority-controlled intersection, refer Appendix E and Appendix F.

The priority-controlled intersection has been designed so that minimal civil work will be required when the intersection is signalised. This has been discussed at the traffic committee.

The kerbside lane in Link Road could be converted to a combined left and right turn lane to increase the capacity of this intersection as most of the traffic will exit to the north from Georges Cove marina precinct. On the south approach, the 60 long merging lane is provided on the interim layout which could be adjusted as part of the signalisation process in due course. After the TfNSW's in principle approval of the traffic signal at this intersection, a traffic signal plan (TCS) will be prepared for TfNSW consideration. In the traffic signal design, this merge lane could be excluded if it is considered unnecessary. Hence, this merge lane could be retained in the interim giveway controlled intersection as it is already approved by the Liverpool traffic committee.

We trust this thoroughly addresses the TfNSW comments and seek TfNSW in principle approval of the traffic signals at Brickmakers Drive/ Link Road intersection. Should you require further clarification regarding this matter, please do not hesitate to contact the undersigned.

Yours sincerely

A handwritten signature in black ink, appearing to read 'Abdullah Uddin', with a stylized, cursive script.

Abdullah Uddin

Associate Traffic Engineer

auddin@emmconsulting.com.au

0425 478 650

Appendix A

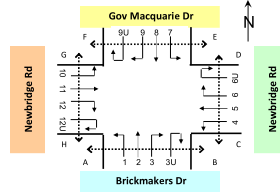
Intersection and tube count data

Job No. : N5560
Client : EMM
Suburb : Moorebank
Location : 1. Newbridge Rd / Gov Macquarie Dr / Brickmakers Dr

Day/Date : Thu, 30th January 2020
Weather : Fine
Description : Classified Intersection Count

: 15 mins Data

Classifications	Class 1	Class 2
Lights		
Heavies		



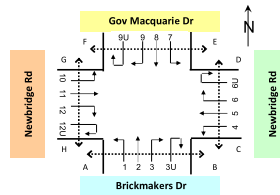
Approach	Brickmakers Dr												Newbridge Rd											
Direction	Direction 1 (Left Turn)			Direction 2 (Right Turn)			Direction 3 (Right Turn)			Direction 3U (U Turn)			Direction 4 (Left Turn)			Direction 5 (Through)			Direction 6 (Right Turn)			Direction 6U (U Turn)		
Time Period	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total
7:00 to 7:15	1	0	1	33	2	35	134	1	135	0	0	0	70	1	71	200	30	230	101	14	115	0	0	0
7:15 to 7:30	2	0	2	41	1	42	124	0	124	0	0	0	60	1	61	181	39	220	113	16	129	0	0	0
7:30 to 7:45	4	0	4	42	0	42	88	0	88	0	0	0	63	0	63	194	26	220	60	6	66	0	0	0
7:45 to 8:00	1	0	1	57	1	58	146	3	149	0	0	0	88	6	94	212	25	237	141	11	152	0	0	0
8:00 to 8:15	3	0	3	72	0	72	161	2	163	0	0	0	65	0	65	164	25	189	125	14	139	0	0	0
8:15 to 8:30	6	0	6	72	1	73	117	1	118	0	0	0	64	1	65	149	23	172	152	12	164	0	0	0
8:30 to 8:45	2	0	2	64	1	65	108	2	110	0	0	0	58	2	60	159	29	188	118	13	131	0	0	0
8:45 to 9:00	3	0	3	49	1	50	109	1	110	0	0	0	64	1	65	196	32	228	97	15	112	0	0	0
AM Totals	22	0	22	430	7	437	987	10	997	0	0	0	532	12	544	1,455	229	1,684	907	101	1,008	0	0	0
16:00 to 16:15	4	0	4	25	1	26	102	4	106	0	0	0	165	2	167	411	19	430	121	12	133	0	0	0
16:15 to 16:30	2	1	3	21	0	21	112	0	112	0	0	0	151	1	152	332	19	351	91	15	106	0	0	0
16:30 to 16:45	2	0	2	24	0	24	75	0	75	0	0	0	146	2	148	306	23	329	92	17	109	0	0	0
16:45 to 17:00	5	0	5	26	0	26	54	1	55	0	0	0	162	1	163	381	24	405	125	11	136	0	0	0
17:00 to 17:15	4	0	4	16	0	16	77	2	79	0	0	0	166	2	168	411	14	425	89	13	102	0	0	0
17:15 to 17:30	4	0	4	18	0	18	52	2	54	0	0	0	139	3	142	468	16	484	114	13	127	1	0	1
17:30 to 17:45	1	0	1	18	0	18	55	0	55	0	0	0	145	0	145	398	17	415	109	12	121	0	0	0
17:45 to 18:00	1	0	1	24	0	24	46	0	46	0	0	0	159	0	159	383	17	400	141	15	156	0	0	0
PM Totals	23	1	24	172	1	173	571	9	580	0	0	0	1,233	11	1,244	3,090	149	3,239	882	108	990	1	0	1

Approach	Gov Macquarie Dr												Newbridge Rd												Crossing Pedestrians									
Direction	Direction 7 (Left Turn)			Direction 8 (Through)			Direction 9 (Right Turn)			Direction 9U (U Turn)			Direction 10 (Left Turn)			Direction 11 (Through)			Direction 12 (Right Turn)			Direction 12U (U Turn)												
Time Period	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	B to A	A to B	D to C	C to D	F to E	E to F	H to G	G to H	Total	
7:00 to 7:15	113	14	127	24	0	24	10	14	24	0	0	0	16	10	26	389	23	412	0	0	0	0	0	0	0	0	0	0	0	0	0	3	1	4
7:15 to 7:30	98	11	109	21	1	22	11	8	19	0	0	0	26	8	34	506	43	549	1	0	1	0	0	0	0	0	0	0	0	0	0	1	0	1
7:30 to 7:45	130	14	144	19	0	19	9	14	23	0	0	0	24	6	30	435	33	468	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	2
7:45 to 8:00	117	9	126	28	1	29	11	12	23	0	0	0	31	11	42	375	32	407	0	0	0	0	0	0	0	0	0	0	0	0	0	3	1	4
8:00 to 8:15	133	16	149	26	5	31	8	10	18	0	0	0	26	17	43	294	32	326	0	1	1	1	0	1	1	1	0	0	0	1	0	6	1	9
8:15 to 8:30	142	16	158	19	0	19	14	13	27	0	0	0	38	11	49	316	36	352	1	0	1	0	0	0	0	0	1	0	1	0	0	8	2	12
8:30 to 8:45	135	22	157	45	1	46	19	15	34	0	0	0	51	10	61	295	44	339	0	1	1	0	0	0	0	0	0	0	0	0	0	4	0	4
8:45 to 9:00	72	26	98	26	2	28	25	11	36	0	0	0	49	10	59	316	49	365	5	0	5	0	0	0	0	0	0	0	1	0	0	2	1	4
AM Totals	940	128	1,068	208	10	218	107	97	204	0	0	0	261	83	344	1,926	292	3,218	7	2	9	1	0	1	1	1	1	0	2	1	0	28	7	40
16:00 to 16:15	194	24	218	64	0	64	41	11	52	0	0	0	2	0	2	1	0	1	0	0	0	0	0	0	1	0	1	0	0	0	0	0	6	8
16:15 to 16:30	197	14	211	59	0	59	35	10	45	0	0	0	6	3	9	178	14	192	1	0	1	1	0	1	1	0	4	0	3	2	0	0	0	10
16:30 to 16:45	140	3	143	63	0	63	31	6	37	0	0	0	23	12	35	351	15	366	0	0	0	0	0	0	0	0	0	0	0	0	1	1	1	3
16:45 to 17:00	105	8	113	51	2	53	20	9	29	0	0	0	25	6	31	209	11	220	1	0	1	0	0	0	0	1	0	1	1	0	3	1	1	8
17:00 to 17:15	153	9	162	79	1	80	42	5	47	0	0	0	27	7	34	276	11	287	3	0	3	0	0	0	0	0	0	0	0	0	1	0	0	1
17:15 to 17:30	109	3	112	67	0	67	46	7	53	0	0	0	25	8	33	308	12	320	2	0	2	0	0	0	0	0	0	0	0	0	0	0	1	1
17:30 to 17:45	109	4	113	53	0	53	29	5	34	0	0	0	14	8	22	311	15	326	2	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0
17:45 to 18:00	80	3	83	37	0	37	26	8	34	0	0	0	24	6	30	234	12	246	2	0	2	0	0	0	0	0	0	1	0	0	0	0	0	1
PM Totals	1,087	68	1,155	473	3	476	270	61	331	0	0	0	146	50	196	1,868	90	1,958	11	0	11	1	0	1	3	0	7	1	3	7	2	9	32	

Job No. : N5560
Client : EMM
Suburb : Moorebank
Location : 1. Newbridge Rd / Gov Macquarie Dr / Brickmakers Dr

Day/Date : Thu, 30th January 2020
Weather : Fine
Description : Classified Intersection Count

: Hourly Summary



Approach	Brickmakers Dr												Newbridge Rd											
Direction	Direction 1 (Left Turn)			Direction 2 (Through)			Direction 3 (Right Turn)			Direction 3U (U Turn)			Direction 4 (Left Turn)			Direction 5 (Through)			Direction 6 (Right Turn)			Direction 6U (U Turn)		
Time Period	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total
7:00 to 8:00	8	0	8	173	4	177	492	4	496	0	0	0	281	8	289	787	120	907	415	47	462	0	0	0
7:15 to 8:15	10	0	10	212	2	214	519	5	524	0	0	0	276	7	283	751	115	866	439	47	486	0	0	0
7:30 to 8:30	14	0	14	243	2	245	512	6	518	0	0	0	280	7	287	719	99	818	478	43	521	0	0	0
7:45 to 8:45	12	0	12	265	3	268	532	8	540	0	0	0	275	9	284	684	102	786	536	50	586	0	0	0
8:00 to 9:00	14	0	14	257	3	260	495	6	501	0	0	0	251	4	255	668	109	777	492	54	546	0	0	0
AM Totals	22	0	22	430	7	437	987	10	997	0	0	0	532	12	544	1,455	229	1,684	907	101	1,008	0	0	0
16:00 to 17:00	13	1	14	96	1	97	341	5	346	0	0	0	624	6	630	1,430	85	1,515	429	55	484	0	0	0
16:15 to 17:15	13	1	14	87	0	87	316	3	319	0	0	0	625	6	631	1,430	80	1,510	397	56	453	0	0	0
16:30 to 17:30	15	0	15	84	0	84	256	5	261	0	0	0	613	8	621	1,566	77	1,643	420	54	474	1	0	1
16:45 to 17:45	14	0	14	78	0	78	238	5	243	0	0	0	612	6	618	1,058	71	1,129	437	49	486	1	0	1
17:00 to 18:00	10	0	10	76	0	76	230	4	234	0	0	0	609	5	614	1,660	64	1,724	453	53	506	1	0	1
PM Totals	23	1	24	172	1	173	571	9	580	0	0	0	1,233	11	1,244	3,090	149	3,239	882	108	990	1	0	1

Direction	Direction 7 (Left Turn)			Direction 8 (Through)			Direction 9 (Right Turn)			Direction 9U (U Turn)			Direction 10 (Left Turn)			Direction 11 (Through)			Direction 12 (Right Turn)			Direction 12U (U Turn)			Pedestrians									
	Lights	Heavy	Total	Lights	Heavy	Total	Lights	Heavy	Total	Lights	Heavy	Total	Lights	Heavy	Total	Lights	Heavy	Total	Lights	Heavy	Total	Lights	Heavy	Total	B to A	A to B	D to C	C to D	F to E	E to F	H to G	G to H	Total	
7:00 to 8:00	458	48	506	92	2	94	41	48	89	0	0	0	97	35	132	1,705	131	1,836	1	0	1	0	0	0	0	0	0	0	0	0	8	3	11	
7:15 to 8:15	478	50	528	94	7	101	39	44	83	0	0	0	107	42	149	1,610	140	1,750	1	1	2	1	0	0	1	0	0	0	1	0	11	3	16	
7:30 to 8:30	522	55	577	92	6	98	42	49	91	0	0	0	119	45	164	1,420	133	1,553	1	1	2	1	0	0	1	1	1	0	1	1	0	18	5	27
7:45 to 8:45	527	63	590	118	7	125	52	50	102	0	0	0	146	49	195	1,280	144	1,424	1	2	3	1	0	1	1	1	1	0	1	1	0	21	4	29
8:00 to 9:00	482	80	562	116	8	124	66	49	115	0	0	0	164	48	212	1,221	161	1,382	6	2	8	1	0	1	1	1	1	0	2	1	0	20	4	29
AM Totals	940	128	1,068	208	10	218	107	97	204	0	0	0	261	83	344	2,926	292	3,218	7	2	9	1	0	1	1	1	1	0	2	1	0	28	7	40
16:00 to 17:00	636	49	685	237	2	239	127	36	163	0	0	0	56	21	77	739	40	779	2	0	2	1	0	1	3	0	6	1	3	7	6	2	8	29
16:15 to 17:15	595	34	629	252	3	255	128	30	158	0	0	0	81	28	109	1,014	51	1,065	5	0	5	1	0	1	2	0	5	1	3	7	2	2	22	
16:30 to 17:30	507	23	530	260	3	263	139	27	166	0	0	0	100	33	133	1,144	49	1,193	6	0	6	0	0	0	1	0	1	1	0	5	2	3	13	
16:45 to 17:45	476	24	500	250	3	253	137	26	163	0	0	0	91	29	120	1,104	49	1,153	8	0	8	0	0	0	1	0	1	1	0	4	1	2	10	
17:00 to 18:00	451	19	470	236	1	237	143	25	168	0	0	0	90	29	119	1,120	50	1,179	9	0	9	0	0	0	0	0	1	0	0	1	0	1	1	3
PM Totals	1,087	68	1,155	473	3	476	270	61	331	0	0	0	146	50	196	1,888	90	1,998	11	0	11	1	0	1	3	0	7	1	3	7	2	9	32	

Job No N5560
Client Eric Lei
Site Brickmakers Drive
Location Mooorebank
Site No 1
Start Date 30-Jan-20
Description Volume Summary
Direction Combined



Hour Starting	Day of Week							W'Day Ave 14929	7 Day Ave 13700
	Mon	Tue	Wed	Thu	Fri	Sat	Sun		
	3-Feb	4-Feb	5-Feb	30-Jan	31-Jan	1-Feb	2-Feb		
AM Peak	1152	1099	1092	1122	1200	882	710		
PM Peak	1217	1228	1259	1313	1203	919	772		
0:00	81	83	79	82	98	167	196	85	112
1:00	42	51	52	34	31	95	103	42	58
2:00	30	25	29	38	37	70	77	32	44
3:00	35	43	46	56	53	72	59	47	52
4:00	141	139	136	148	141	86	60	141	122
5:00	460	454	443	448	432	188	100	447	361
6:00	800	853	850	828	806	357	161	827	665
7:00	1030	879	1086	996	994	433	255	997	810
8:00	1152	1099	1092	1122	1200	609	343	1133	945
9:00	778	811	942	785	795	772	513	822	771
10:00	579	598	624	631	669	882	653	620	662
11:00	551	634	593	674	670	838	710	624	667
12:00	600	700	671	616	722	919	772	662	714
13:00	657	712	727	735	782	795	736	723	735
14:00	970	983	963	1076	1012	724	651	1001	911
15:00	1217	1187	1189	1306	1203	646	674	1220	1060
16:00	1162	1228	1135	1313	1179	615	691	1203	1046
17:00	1211	1197	1259	1180	1182	651	642	1206	1046
18:00	969	1024	1039	975	931	655	593	988	884
19:00	666	666	662	661	701	541	420	671	617
20:00	497	520	553	568	536	451	400	535	504
21:00	343	445	478	446	486	444	325	440	424
22:00	231	289	278	292	363	399	253	291	301
23:00	152	128	168	136	280	317	143	173	189
Total	14354	14748	15094	15146	15303	11726	9530	14929	13700

7-19	10876	11052	11320	11409	11339	8539	7233	11199	10253
6-22	13182	13536	13863	13912	13868	10332	8539	13672	12462
6-24	13565	13953	14309	14340	14511	11048	8935	14136	12952
0-24	14354	14748	15094	15146	15303	11726	9530	14929	13700

Appendix B

SIDRA results (existing)

MOVEMENT SUMMARY



Site: 101 [Ex Newbridge Rd/Gov Macquarie Dr/Brickmakers Dr AM]

Existing Four Way Intersection

Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 140 seconds (Site User-Given Phase Times)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Back of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South: Brickmakers Drive												
1	L2	8	0.0	0.750	71.4	LOS F	12.8	91.2	1.00	0.88	1.09	25.1
2	T1	177	2.3	0.750	66.9	LOS E	12.8	91.2	1.00	0.88	1.09	25.1
3	R2	496	0.8	0.990	110.2	LOS F	22.7	160.1	1.00	1.17	1.58	18.5
Approach		681	1.2	0.990	98.5	LOS F	22.7	160.1	1.00	1.09	1.45	19.9
East: Newbridge Road												
4	L2	289	2.8	0.242	9.2	LOS A	3.4	24.4	0.30	0.67	0.30	51.6
5	T1	907	13.2	0.268	12.2	LOS A	9.0	70.2	0.48	0.42	0.48	56.8
6	R2	462	10.2	1.198	158.8	LOS F	33.1	252.1	1.00	1.18	1.89	15.2
Approach		1658	10.6	1.198	52.5	LOS D	33.1	252.1	0.59	0.67	0.84	31.5
North: Governor Macquarie Drive												
7	L2	506	9.5	0.497	37.1	LOS C	10.4	78.4	0.88	0.85	0.99	36.8
8	T1	94	2.1	0.380	62.6	LOS E	6.0	42.7	0.96	0.76	0.96	25.8
9	R2	89	53.9	0.366	47.9	LOS D	4.7	47.8	0.93	0.77	0.93	30.0
Approach		689	14.2	0.497	42.0	LOS C	10.4	78.4	0.89	0.83	0.98	34.1
West: Newbridge Road												
10	L2	132	26.5	0.873	55.7	LOS D	43.9	336.6	0.99	0.98	1.23	33.4
11	T1	1836	7.1	0.873	48.4	LOS D	45.4	337.3	0.99	0.97	1.12	36.3
12	R2	1	0.0	0.001	32.0	LOS C	0.0	0.3	0.61	0.61	0.61	36.7
Approach		1969	8.4	0.873	48.9	LOS D	45.4	337.3	0.99	0.97	1.13	36.1
All Vehicles		4997	8.9	1.198	55.9	LOS D	45.4	337.3	0.85	0.87	1.06	31.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians								
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back of Queue Pedestrian ped	Back of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	21	16.0	LOS B	0.0	0.0	0.48	0.48
P2	East Full Crossing	11	64.2	LOS F	0.0	0.0	0.96	0.96
P3	North Full Crossing	21	35.7	LOS D	0.1	0.1	0.71	0.71
P4	West Full Crossing	53	64.3	LOS F	0.2	0.2	0.96	0.96
All Pedestrians		105	48.9	LOS E			0.81	0.81

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY



Site: 101 [Ex Newbridge Rd/Gov Macquarie Dr/Brickmakers Dr PM]

Existing Four Way Intersection

Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 140 seconds (Site User-Given Phase Times)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Back of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South: Brickmakers Drive												
1	L2	15	0.0	0.326	61.5	LOS E	6.1	42.4	0.93	0.74	0.93	27.2
2	T1	84	0.0	0.326	56.9	LOS E	6.1	42.4	0.93	0.74	0.93	27.3
3	R2	261	1.9	0.712	73.9	LOS F	9.1	64.8	1.00	0.85	1.09	23.8
Approach		360	1.4	0.712	69.4	LOS E	9.1	64.8	0.98	0.82	1.05	24.7
East: Newbridge Road												
4	L2	621	1.3	0.628	19.7	LOS B	20.4	144.6	0.64	0.82	0.74	43.6
5	T1	1643	4.7	0.509	13.5	LOS A	21.9	159.6	0.55	0.50	0.55	55.7
6	R2	474	11.4	0.942	63.6	LOS E	18.4	141.5	0.99	0.98	1.39	30.3
Approach		2738	5.1	0.942	23.5	LOS B	21.9	159.6	0.65	0.65	0.74	46.2
North: Governor Macquarie Drive												
7	L2	530	4.3	0.404	34.2	LOS C	10.2	73.9	0.79	0.85	0.96	38.6
8	T1	263	1.1	0.865	72.5	LOS F	19.4	137.3	1.00	0.98	1.22	23.8
9	R2	166	16.3	0.511	49.3	LOS D	9.2	73.5	0.92	0.79	0.92	32.5
Approach		959	5.5	0.865	47.3	LOS D	19.4	137.3	0.87	0.88	1.03	32.7
West: Newbridge Road												
10	L2	133	24.8	0.625	42.6	LOS D	22.8	174.0	0.87	0.81	0.98	37.7
11	T1	1193	4.1	0.625	37.2	LOS C	24.7	179.2	0.87	0.78	0.90	40.8
12	R2	6	0.0	0.008	34.8	LOS C	0.2	1.7	0.64	0.65	0.64	35.5
Approach		1332	6.2	0.625	37.8	LOS C	24.7	179.2	0.87	0.78	0.91	40.4
All Vehicles		5389	5.2	0.942	34.4	LOS C	24.7	179.2	0.77	0.74	0.85	39.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians								
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back of Queue Pedestrian ped	Back of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	21	15.1	LOS B	0.0	0.0	0.46	0.46
P2	East Full Crossing	11	64.2	LOS F	0.0	0.0	0.96	0.96
P3	North Full Crossing	21	38.7	LOS D	0.1	0.1	0.74	0.74
P4	West Full Crossing	53	64.3	LOS F	0.2	0.2	0.96	0.96
All Pedestrians		105	49.3	LOS E			0.82	0.82

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Appendix C

SIDRA results (signalisation)

MOVEMENT SUMMARY

 Site: 101 [Dev Newbridge Rd/Gov Macquarie Dr/Brickmakers Dr AM]

 Network: N101 [Dev AM]

Dev Four Way Intersection

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 140 seconds (Network Site User-Given Phase Times)

Movement Performance - Vehicles														
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back of Queue Vehicles	Prop. Distance	Effective Stop Rate	Aver. No. Cycles	Average Speed	
		veh/h	%	veh/h	%	v/c	sec		veh	m			km/h	
South: Brickmakers Drive														
1	L2	144	6.3	144	6.3	0.689	51.7	LOS D	22.8	164.1	0.95	0.83	0.95	29.0
2	T1	233	1.7	233	1.7	0.689	47.1	LOS D	22.8	164.1	0.95	0.83	0.95	29.6
3	R2	622	2.3	622	2.3	1.254	303.2	LOS F	50.1	357.6	1.00	1.74	2.58	8.1
Approach		999	2.7	999	2.7	1.254	207.2	LOS F	50.1	357.6	0.98	1.40	1.96	11.1
East: Newbridge Road														
4	L2	336	5.7	336	5.7	0.334	11.7	LOS A	5.9	43.4	0.42	0.70	0.42	48.9
5	T1	907	13.2	907	13.2	0.268	12.2	LOS A	9.0	70.2	0.48	0.42	0.48	56.8
6	R2	462	10.2	462	10.2	1.198	158.8	LOS F	33.1	252.1	1.00	1.18	1.89	15.2
Approach		1705	10.9	1705	10.9	1.198	51.8	LOS D	33.1	252.1	0.61	0.68	0.85	31.0
North: Governor Macquarie Drive														
7	L2	506	9.5	506	9.5	0.497	37.1	LOS C	10.4	78.4	0.88	0.85	0.99	36.8
8	T1	143	1.4	143	1.4	0.647	67.2	LOS E	9.6	68.3	1.00	0.82	1.02	19.3
9	R2	89	53.9	89	53.9	0.818	83.5	LOS F	6.8	69.8	1.00	0.95	1.33	23.2
Approach		738	13.3	738	13.3	0.818	48.6	LOS D	10.4	78.4	0.91	0.86	1.04	31.2
West: Newbridge Road														
10	L2	132	26.5	132	26.5	0.910	63.5	LOS E	50.1	383.5	1.00	1.02	1.16	31.2
11	T1	1836	7.1	1836	7.1	0.910	56.1	LOS D	51.6	383.4	0.98	1.02	1.15	33.8
12	R2	125	8.0	125	8.0	0.170	34.6	LOS C	5.3	40.0	0.67	0.75	0.67	30.6
Approach		2093	8.4	2093	8.4	0.910	55.3	LOS D	51.6	383.5	0.97	1.00	1.12	33.5
All Vehicles		5535	8.8	5535	8.8	1.254	80.7	LOS F	51.6	383.5	0.85	0.96	1.18	24.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians									
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back of Queue Pedestrian ped	Prop. Distance m	Effective Stop Rate		
P1	South Full Crossing	1	16.0	LOS B	0.0	0.0	0.48	0.48	
P2	East Full Crossing	1	53.2	LOS E	0.0	0.0	0.87	0.87	
P3	North Full Crossing	1	35.7	LOS D	0.0	0.0	0.71	0.71	
P4	West Full Crossing	12	50.6	LOS E	0.0	0.0	0.85	0.85	
All Pedestrians		15	47.3	LOS E			0.82	0.82	

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)
Pedestrian movement LOS values are based on average delay per pedestrian movement.
Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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MOVEMENT SUMMARY

 Site: 102 [Dev Brickmakers Dr/Link Rd AM]

 Network: N101 [Dev AM]

New Intersection with Traffic Signals

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 50 seconds (Site User-Given Cycle Time)

Movement Performance - Vehicles														
Mov ID	Turn	Demand Flows Total	Arrival Flows HV	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back of Queue Vehicles	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed		
		veh/h	%	veh/h	%	v/c	sec	veh	m				km/h	
South: Brickmakers Drive														
2	T1	656	1.8	656	1.8	0.661	14.8	LOS B	8.6	60.9	0.87	0.79	0.93	35.5
3	R2	66	3.0	66	3.0	0.661	20.1	LOS B	7.1	50.4	0.88	0.81	0.95	40.4
Approach		722	1.9	722	1.9	0.661	15.3	LOS B	8.6	60.9	0.87	0.80	0.93	36.2
East: Link Road														
4	L2	133	0.8	133	0.8	0.095	5.4	LOS A	0.4	3.0	0.27	0.60	0.27	46.0
6	R2	319	6.0	319	6.0	0.629	20.0	LOS B	6.9	50.6	0.88	0.83	0.91	32.5
Approach		452	4.4	452	4.4	0.629	15.7	LOS B	6.9	50.6	0.70	0.77	0.72	37.5
North: Brickmakers Drive														
7	L2	220	9.5	220	9.5	0.166	5.6	LOS A	0.8	5.8	0.29	0.61	0.29	44.9
8	T1	340	9.7	340	9.7	0.463	10.9	LOS A	5.3	40.1	0.66	0.56	0.66	42.1
Approach		560	9.6	560	9.6	0.463	8.8	LOS A	5.3	40.1	0.51	0.58	0.51	43.2
All Vehicles		1734	5.1	1734	5.1	0.661	13.3	LOS A	8.6	60.9	0.71	0.72	0.74	39.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians									
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back of Queue Pedestrian ped	Prop. Queued	Effective Stop Rate		
P1	South Full Crossing	53	17.7	LOS B	0.1	0.1	0.84	0.84	
P2	East Full Crossing	53	13.7	LOS B	0.1	0.1	0.74	0.74	
P3	North Full Crossing	53	17.7	LOS B	0.1	0.1	0.84	0.84	
All Pedestrians		158	16.4	LOS B			0.81	0.81	

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY

 Site: 101 [Dev Newbridge Rd/Gov Macquarie Dr/Brickmakers Dr PM]

 Network: N102 [Dev PM]

Dev Four Way Intersection

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 140 seconds (Network Site User-Given Phase Times)

Movement Performance - Vehicles														
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back of Queue Vehicles	Distance	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South: Brickmakers Drive														
1	L2	153	4.6	153	4.6	0.554	50.1	LOS D	17.1	122.2	0.90	0.80	0.90	29.4
2	T1	143	0.0	143	0.0	0.554	45.5	LOS D	17.1	122.2	0.90	0.80	0.90	29.8
3	R2	382	3.1	382	3.1	1.051	145.6	LOS F	20.2	145.2	1.00	1.31	1.86	14.8
Approach		678	2.8	678	2.8	1.051	103.0	LOS F	20.2	145.2	0.96	1.09	1.44	19.0
East: Newbridge Road														
4	L2	693	2.0	693	2.0	1.080	146.5	LOS F	79.3	564.7	1.00	1.22	1.80	10.0
5	T1	1643	4.7	1643	4.7	0.505	13.4	LOS A	21.7	157.7	0.55	0.50	0.55	55.8
6	R2	474	11.4	474	11.4	0.942	63.6	LOS E	18.4	141.5	0.99	0.98	1.39	30.3
Approach		2810	5.2	2810	5.2	1.080	54.7	LOS D	79.3	564.7	0.74	0.76	1.00	30.6
North: Governor Macquarie Drive														
7	L2	530	4.3	530	4.3	0.404	34.2	LOS C	10.2	73.9	0.79	0.85	0.96	38.6
8	T1	333	0.9	333	0.9	1.202	256.7	LOS F	49.3	347.7	1.00	1.71	2.38	6.4
9	R2	166	16.3	166	16.3	1.002	125.4	LOS F	16.4	130.7	1.00	1.16	1.73	19.4
Approach		1029	5.2	1029	5.2	1.202	120.9	LOS F	49.3	347.7	0.89	1.18	1.54	17.6
West: Newbridge Road														
10	L2	133	24.8	133	24.8	0.653	45.1	LOS D	24.8	188.8	0.89	0.80	0.89	36.8
11	T1	1193	4.1	1193	4.1	0.653	37.8	LOS C	26.2	190.1	0.87	0.78	0.87	40.5
12	R2	170	3.5	170	3.5	0.242	38.1	LOS C	7.8	56.4	0.73	0.77	0.73	28.9
Approach		1496	5.9	1496	5.9	0.653	38.5	LOS C	26.2	190.1	0.86	0.78	0.86	39.2
All Vehicles		6013	5.1	6013	5.1	1.202	67.4	LOS E	79.3	564.7	0.82	0.87	1.11	27.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians									
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back of Queue Pedestrian	Prop. Distance m	Effective Stop Rate		
P1	South Full Crossing	1	15.1	LOS B	0.0	0.0	0.46	0.46	
P2	East Full Crossing	2	54.0	LOS E	0.0	0.0	0.88	0.88	
P3	North Full Crossing	5	38.6	LOS D	0.0	0.0	0.74	0.74	
P4	West Full Crossing	5	51.4	LOS E	0.0	0.0	0.86	0.86	
All Pedestrians		14	44.1	LOS E			0.79	0.79	

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)
Pedestrian movement LOS values are based on average delay per pedestrian movement.
Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Project: T:\Jobs\2018\J180179 - Georges Cove Marina 2018 EIS\Technical studies\Transport\2020\SIDRA v5.sip8

MOVEMENT SUMMARY

 Site: 102 [Dev Brickmakers Dr/Link Rd PM]

 Network: N102 [Dev PM]

New Intersection with Traffic Signals

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 50 seconds (Site User-Given Cycle Time)

Movement Performance - Vehicles														
Mov ID	Turn	Demand Flows Total	Arrival Flows HV	Flows Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back of Queue Vehicles	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed	
		veh/h	%	veh/h	%	v/c	sec		veh	m			km/h	
South: Brickmakers Drive														
2	T1	396	2.0	396	2.0	0.367	6.6	LOS A	5.4	38.5	0.59	0.51	0.59	42.4
3	R2	122	0.8	122	0.8	0.484	19.2	LOS B	2.5	17.8	0.82	0.78	0.82	39.3
Approach		518	1.7	518	1.7	0.484	9.6	LOS A	5.4	38.5	0.64	0.57	0.64	41.2
East: Link Road														
4	L2	92	2.2	92	2.2	0.066	5.4	LOS A	0.3	2.1	0.26	0.60	0.26	46.0
6	R2	317	4.4	317	4.4	0.895	36.7	LOS C	10.0	72.5	1.00	1.12	1.63	25.1
Approach		409	3.9	409	3.9	0.895	29.7	LOS C	10.0	72.5	0.83	1.00	1.32	30.1
North: Brickmakers Drive														
7	L2	307	3.9	290	4.0	0.252	5.8	LOS A	1.4	10.3	0.33	0.60	0.33	45.2
8	T1	851	8.1	771	8.8	0.711	6.1	LOS A	10.2	76.6	0.57	0.54	0.59	45.2
Approach		1158	7.0	1060 ^{N1}	7.5	0.711	6.0	LOS A	10.2	76.6	0.50	0.56	0.52	45.2
All Vehicles		2085	5.1	1987 ^{N1}	5.3	0.895	11.8	LOS A	10.2	76.6	0.61	0.65	0.72	40.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

^{N1} Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

Movement Performance - Pedestrians								
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back of Queue Pedestrian ped	Prop. Queued	Effective Stop Rate	
P1	South Full Crossing	53	19.4	LOS B	0.1	0.1	0.88	0.88
P2	East Full Crossing	53	8.4	LOS A	0.0	0.0	0.58	0.58
P3	North Full Crossing	53	19.4	LOS B	0.1	0.1	0.88	0.88
All Pedestrians		158	15.7	LOS B			0.78	0.78

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Appendix D

RMS Traffic Signal Warrant Guidelines

2.1 INTRODUCTION

This section describes the general warrants for the installation of traffic signals. It must be emphasised that these are only a guide. If a site satisfies the warrants, it does not necessarily mean that traffic signals are the best solution. All traffic data should be analysed and alternative treatments considered to determine the optimum solution (see Section 4 of the *Road Design Guide*). Traffic signals are sometimes installed due to public pressure or an administrative directive irrespective of the general warrants.

2.2 FACTORS INFLUENCING THE PROVISION OF TRAFFIC SIGNALS

Traffic signals are usually installed at an intersection:

- to provide traffic control at a site with a traffic capacity or road safety problem
- to control conflicting movements with high traffic flows
- to facilitate access to and from local areas in a major/minor road system, including pedestrian movements
- as part of an area wide system of traffic management

A side effect of signalisation is that the traffic flow on a major road is broken up into platoons. This assists nearby pedestrians to cross the major road and vehicles in nearby side streets to cross or enter the major road.

Factors influencing the provision of traffic signals include:

- traffic flows
- traffic conflicts
- traffic accident statistics
- pedestrian requirements
- access to major roads
- cost of installation
- availability of funds
- maintenance costs
- practicality
- feasibility
- the signposted speed limit is not more than 80km/h

General warrants are given in the following sub-sections. The figures stated should only be used as a guide and each intersection should be considered in more detail before being accepted for signal design.

2.3 SIGNALISED INTERSECTIONS

As a guide, a signalised intersection may be considered if one of the following warrants is met.

(a) Traffic demand:

For each of four one-hour periods of an average day:

- (i) the major road flow exceeds 600 vehicles/hour in each direction; and
- (ii) the minor road flow exceeds 200 vehicles/hour in one direction.

OR

(b) Continuous traffic:

For each of four one-hour periods of an average day:

- (i) the major road flow exceeds 900 vehicles/hour in each direction; and
- (ii) the minor road flow exceeds 100 vehicles/hour in one direction; and
- (iii) the speed of traffic on the major road or limited sight distance from the minor road causes undue delay or hazard to the minor road vehicles; and
- (iv) there is no other nearby traffic signal site easily accessible to the minor road vehicles.

OR

(c) Pedestrian safety:

For each of four one-hour periods of an average day:

- (i) the pedestrian flow crossing the major road exceeds 150 persons/hour; and
- (ii) the major road flow exceeds 600 vehicles/hour in each direction or, where there is a central median of at least 1.2 m wide, 1000 vehicles/hour in each direction.

OR

(d) Pedestrian safety – high speed road:

For each of four one-hour periods of an average day:

- (i) the pedestrian flow crossing the major road exceeds 150 persons/hour; and
- (ii) the major road flow exceeds 450 vehicles/hour in each direction or, where there is a central median of at least 1.2 m wide, 750 vehicles/hour in each direction; and
- (iii) the 85th percentile speed on the major road exceeds 75 km/h.

OR

(e) Crashes:

- (i) The intersection has been the site of an average of three or more reported tow-away or casualty traffic accidents per year over a three year period, where the traffic accidents could have been prevented by traffic signals; and
- (ii) the traffic flows are at least 80% of the appropriate flow warrants.

2.4 SIGNALISED MARKED FOOT CROSSINGS AT INTERSECTIONS

A signalised marked foot crossing must be provided on each leg of a signalised intersection (including T Junctions), in a built-up area, except in the following circumstances:

- (a) There are significant road safety implications:
 - (i) there is insufficient sight distance (see Section 4 of the *Road Design Guide*); or
 - (ii) there is adverse road geometry (see Section 4 of the *Road Design Guide*).
- (b) There are significant adverse transport efficiency implications
 - (i) there is an unacceptable increase in delay and degree of saturation which must be substantiated by intersection modelling; or

Liverpool Traffic Committee Minutes

ITEM 10	146 NEWBRIDGE ROAD, MOOREBANK – PROPOSED LINEMARKING AND SIGNPOSTING SCHEME
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INTRODUCTION

The developer of Georges Cove Marina at 146 Newbridge Road, Moorebank (legally known as Lot 7 in DP 1065574) has submitted a signs and linemarking scheme of a planned access road off Brickmakers Drive.

The Committee is requested to support the proposed signs and linemarking scheme as shown in Attachment 10.1 (in the attachment booklet).

ASSESSMENT

The proposed development of 146 Newbridge Road (Georges Cove Marina) will result in a daily average traffic movement of approximately 1300 vehicles. The development's only vehicular access is off Brickmakers Drive.

Access to Georges Cove Marina will be from the existing signalised intersection of Newbridge Road and Brickmakers Drive and via a proposed elevated road bridge which will connect to a road that connects to Brickmakers Drive, forming a 'T' intersection.

The new intersection off Brickmakers Drive will be approximately 300m south of Newbridge Road. This intersection would also provide access to other future developments in the south-eastern corner of Newbridge Road and Brickmakers Drive.

Traffic assessment carried out to date indicates that, in future, the intersection would meet the warrant for traffic signals and would need to be signalised to permit safe and efficient turning movements into and out of the development's access road.

The interim 'T' intersection will be a priority control intersection which would have dedicated right and left turn lanes out of the proposed access road, as well as dedicated left and right turn lanes along Brickmakers Drive.

The Committee is requested to support the interim channelised intersection treatment as shown in Attachment 10.1 (in the attachment booklet).

In addition, the developer of Georges Cove Marina will seek RMS' in-principal approval for the intersection to be signalised in order to accommodate the future traffic volumes.

RECOMMENDATION

That:

1. The Committee supports the proposed signs and linemarking scheme for an access road off Brickmakers Drive, as shown in Attachment 10.1.

COMMITTEE DISCUSSION

The Committee was advised that the proposed linemarking and signposting scheme at the intersection of Brickmakers Drive and the proposed east-west access road, to provide access to the development precinct east of Brickmakers Drive (including the marina and other urban developments) is an interim intersection treatment.

This intersection will be upgraded to traffic signals in future when it meets the RMS traffic signals warrant. As part of the interim intersection layout, the developer is to seek RMS in-principle approval of the traffic signals and include required conduits in the layout.

The Committee discussed and supported the signs and linemarking scheme as an interim treatment until it is upgraded to traffic signals in future.

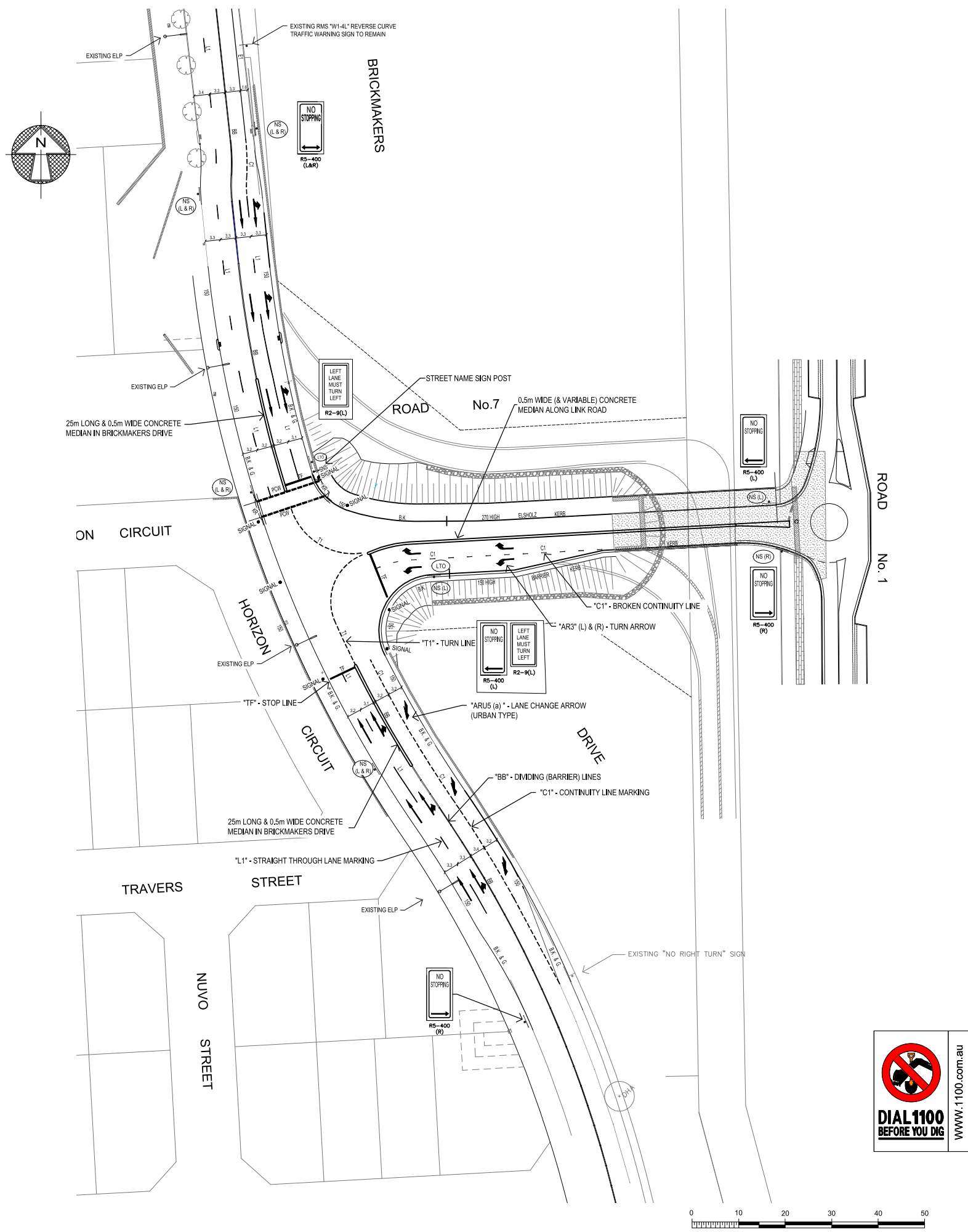
COMMITTEE RECOMMENDATION

That:

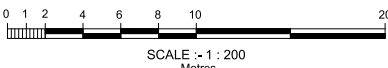
1. Council approves the linemarking and signposting scheme at the Brickmakers Drive and the new road providing access to the development precinct east of Brickmakers Drive (including the marina) as an interim intersection treatment.


Appendix F

Approved Line Marking Plans

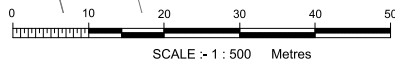
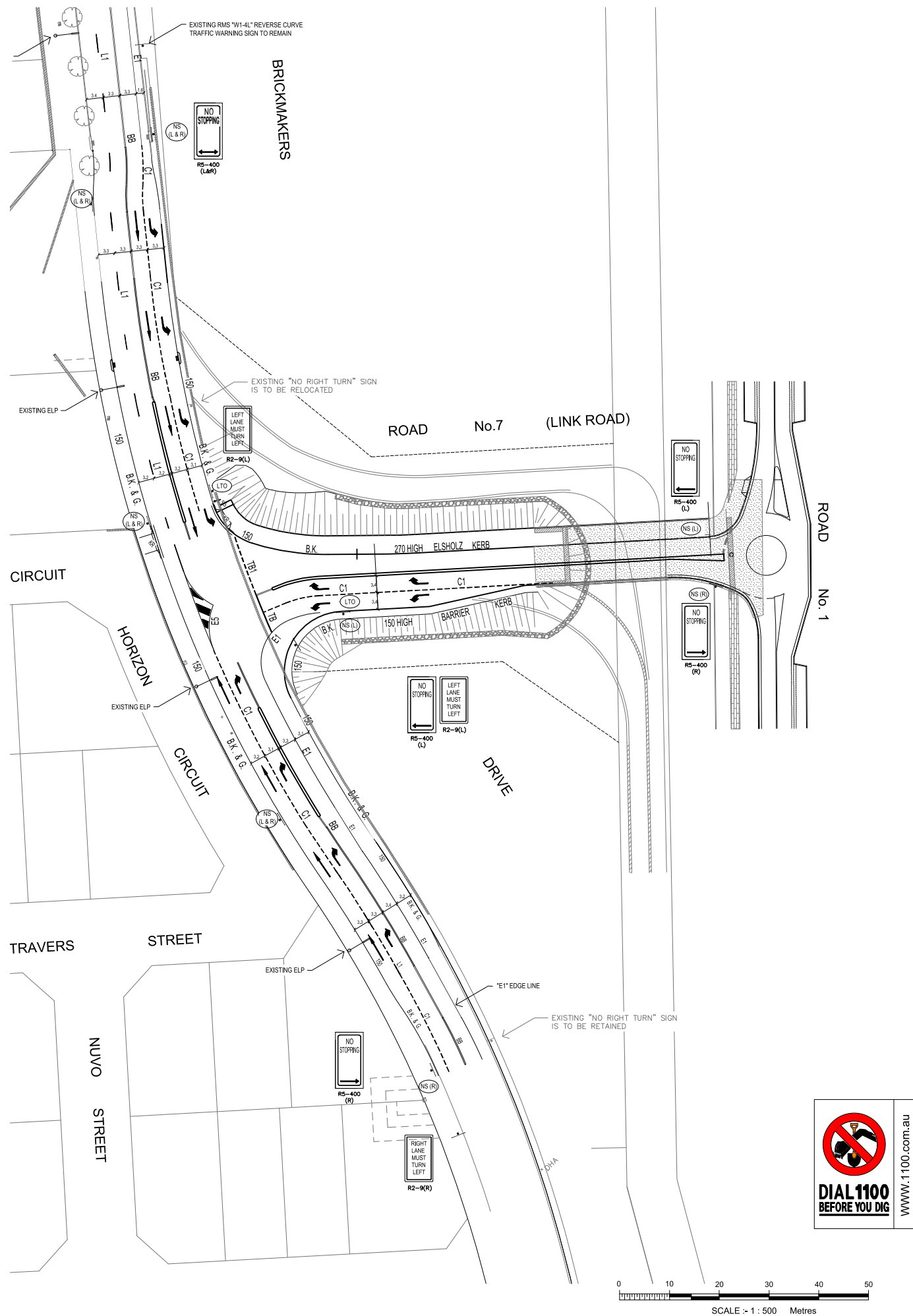
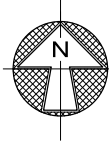
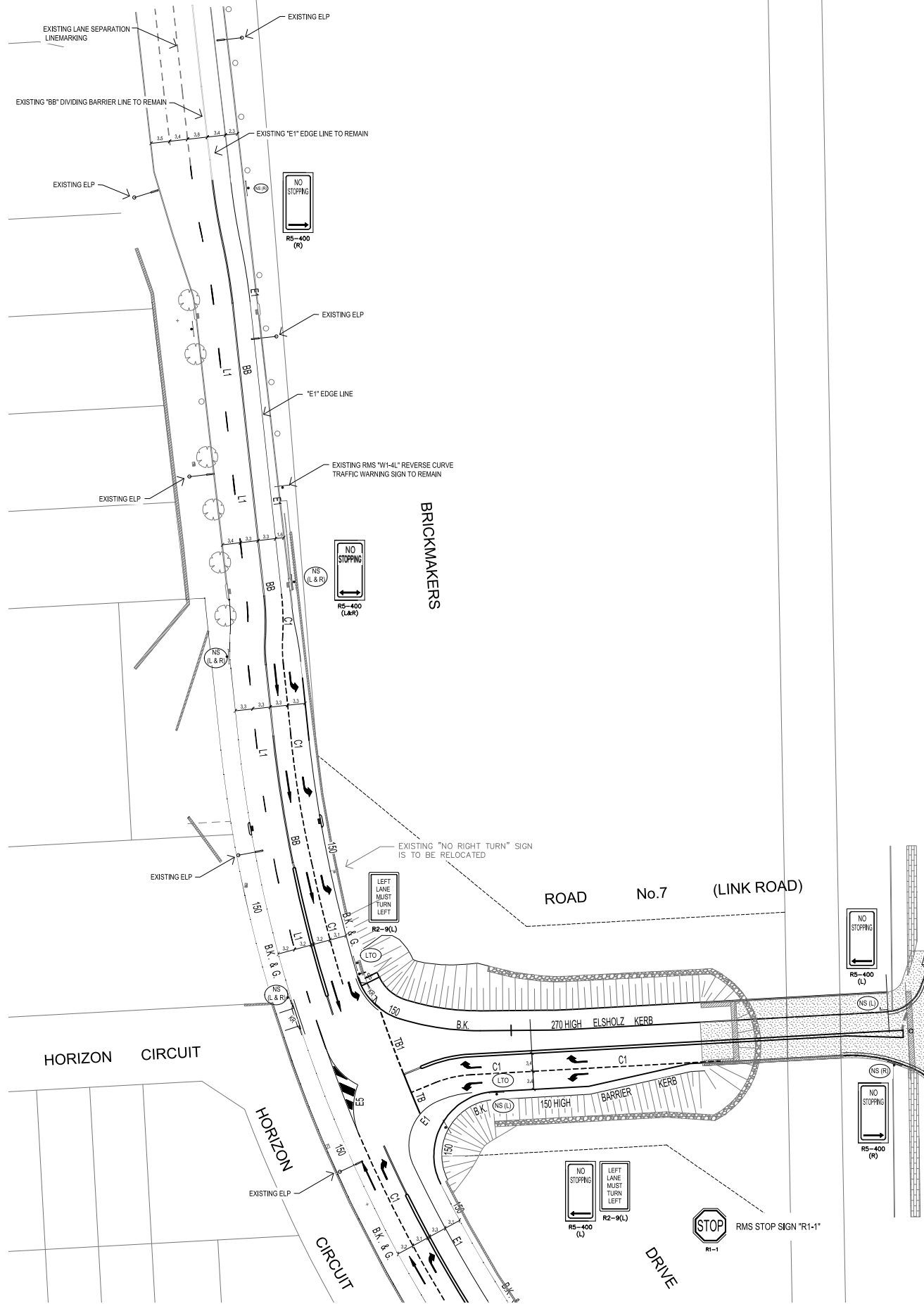



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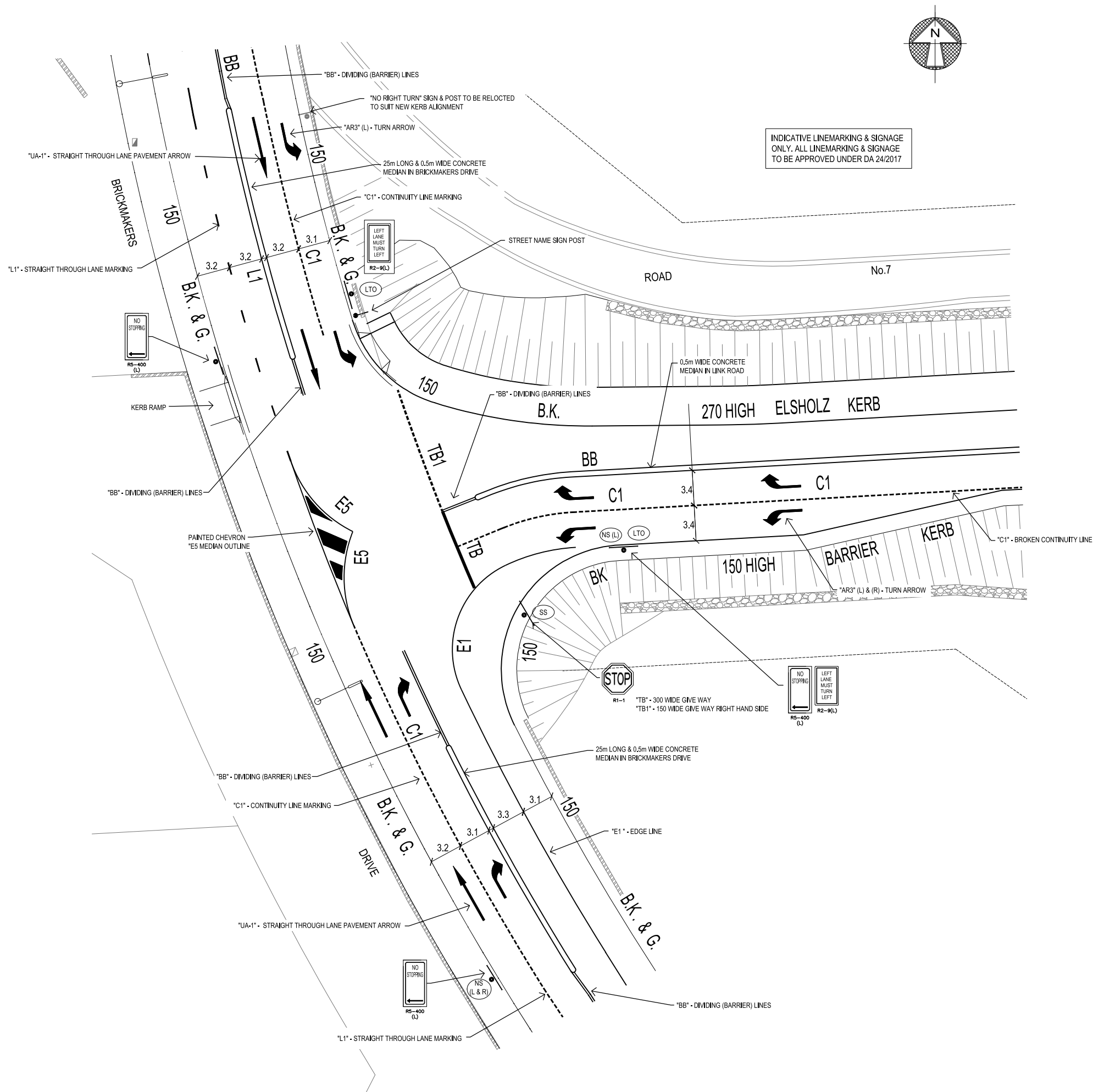
Issue	App	Date	Description	Client: MIRVAC HOMES (NSW) PTY LTD.	Datum : AHD	 <p>John M. Daly & Associates PTY LTD A.B.N. 88051977989</p> <p>Surveying Engineering Project Management Licensed Water Service Co ordinators</p> <p>32 Iolanthe Street P.O. BOX 25 CAMPBELLTOWN N.S.W. 2560</p> <p>PH. (02) 4625 5055 FAX (02) 4628 2013 email: admin@jmd.com.au</p>	Project : MOOREBANK COVE NEWBRIDGE ROAD, MOOREBANK			Sheet 2 of 6 sheets
				Designed by : S.G.	Origin of Levels :		LINEMARKING PLAN FOR FUTURE TRAFFIC LIGHTS INSTALLATION Sheet 2 of 2			Ref: 14005E27
B	S.G.	30-05-2019	ISSUED FOR TRAFFIC COMMITTEE APPROVAL	Date of Drawing : 08-08-2018						
A	S.G.	08-08-2018	ISSUED FOR DA APPROVAL	Ratio (A1) : 1:200	Date of Survey : N/A		Locality : MOOREBANK			L.G.A. : LIVERPOOL
						CAD Ref: S:\JCRS\14\14005\14005 EMW\ENG\CAD\E27 - BRICKMAKERS TRAFFIC\14005E27 LINEMARKING TRAFFIC LIGHTS				

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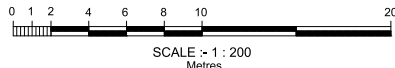


Issue	App	Date	Description	Client: MIRVAC HOMES (NSW) PTY LTD.	Datum : AHD		John M. Daly & Associates PTY LTD A.B.N. 88051977989 Surveying Engineering Project Management Licensed Water Service Co ordinators 32 Ianthe Street P.O. BOX 25 CAMPBELLTOWN N.S.W. 2560 email: admin@jmd.com.au PH. (02) 4625 5055 FAX (02) 4628 2013	Project : MOOREBANK COVE NEWBRIDGE ROAD, MOOREBANK			Sheet 3 of 6 sheets	
				Designed by : S.G.	Origin of Levels :			LINEMARKING PLAN FOR INITIAL INTERSECTION TREATMENT - STOP SIGN			Ref: 14005E27	
				Date of Drawing : 08-08-2018	Date of Survey : N/A			Sheet 1 of 2				
B	S.G.	30-05-2019	ISSUED FOR TRAFFIC COMMITTEE APPROVAL	Ratio (A1) : 1:200				Locality : MOOREBANK			L.G.A. : LIVERPOOL	CAD Ref: S:\J085\14\14005\14005 EMM\ENG\CAD\E27 - BRICKMAKERS TRAFFIC\14005E27 LINEMARKING STOP SIGN
A	S.G.	08-08-2018	ISSUED FOR DA APPROVAL									

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INDICATIVE LINEMARKING & SIGNAGE
ONLY. ALL LINEMARKING & SIGNAGE
TO BE APPROVED UNDER DA 24/2017

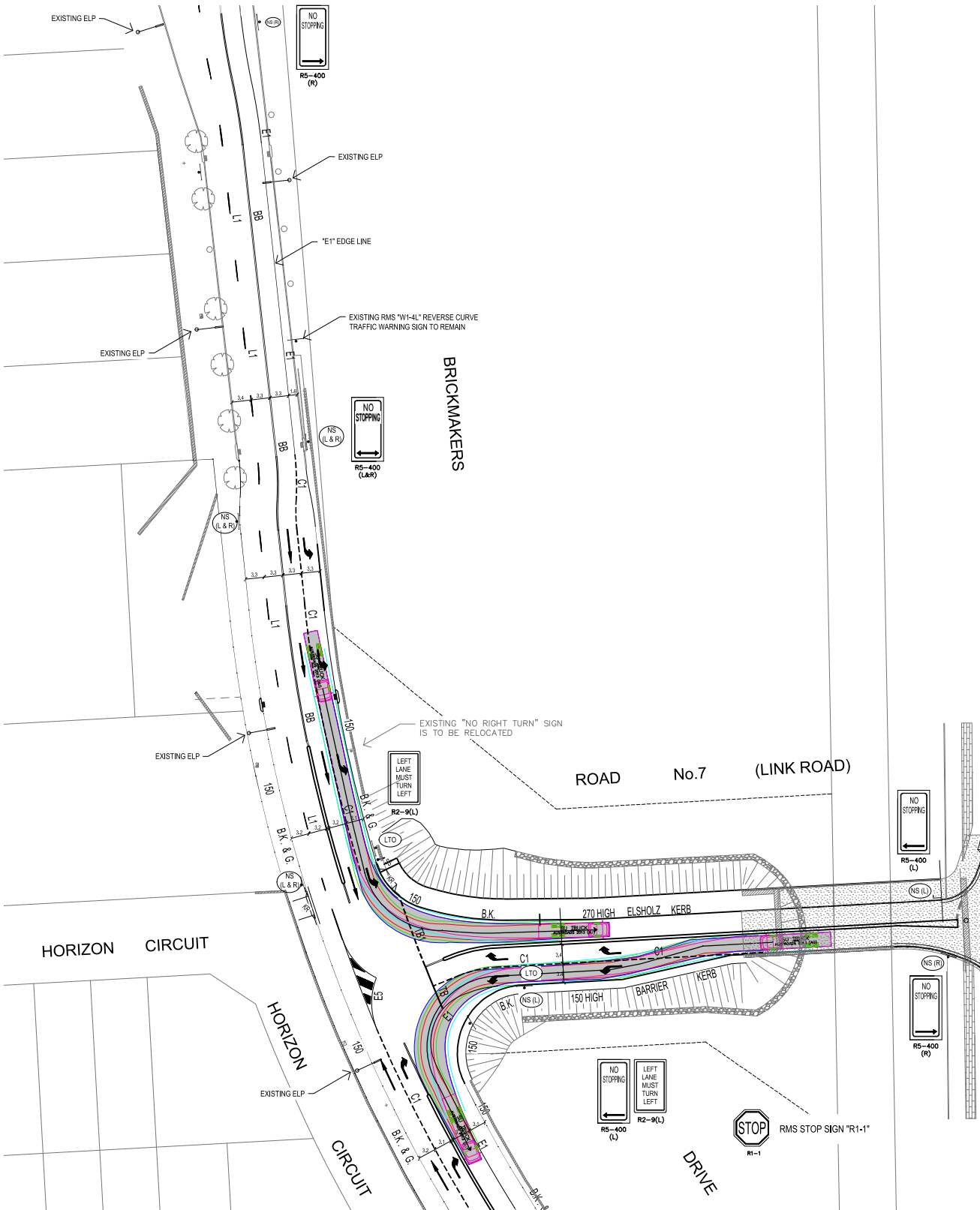


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				Designed by : S.G.	Origin of Levels :				
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B	S.G.	30-05-2019	ISSUED FOR TRAFFIC COMMITTEE APPROVAL	Ratio (A1) : 1:200	Date of Survey : N/A				
A	S.G.	08-08-2018	ISSUED FOR DA APPROVAL						

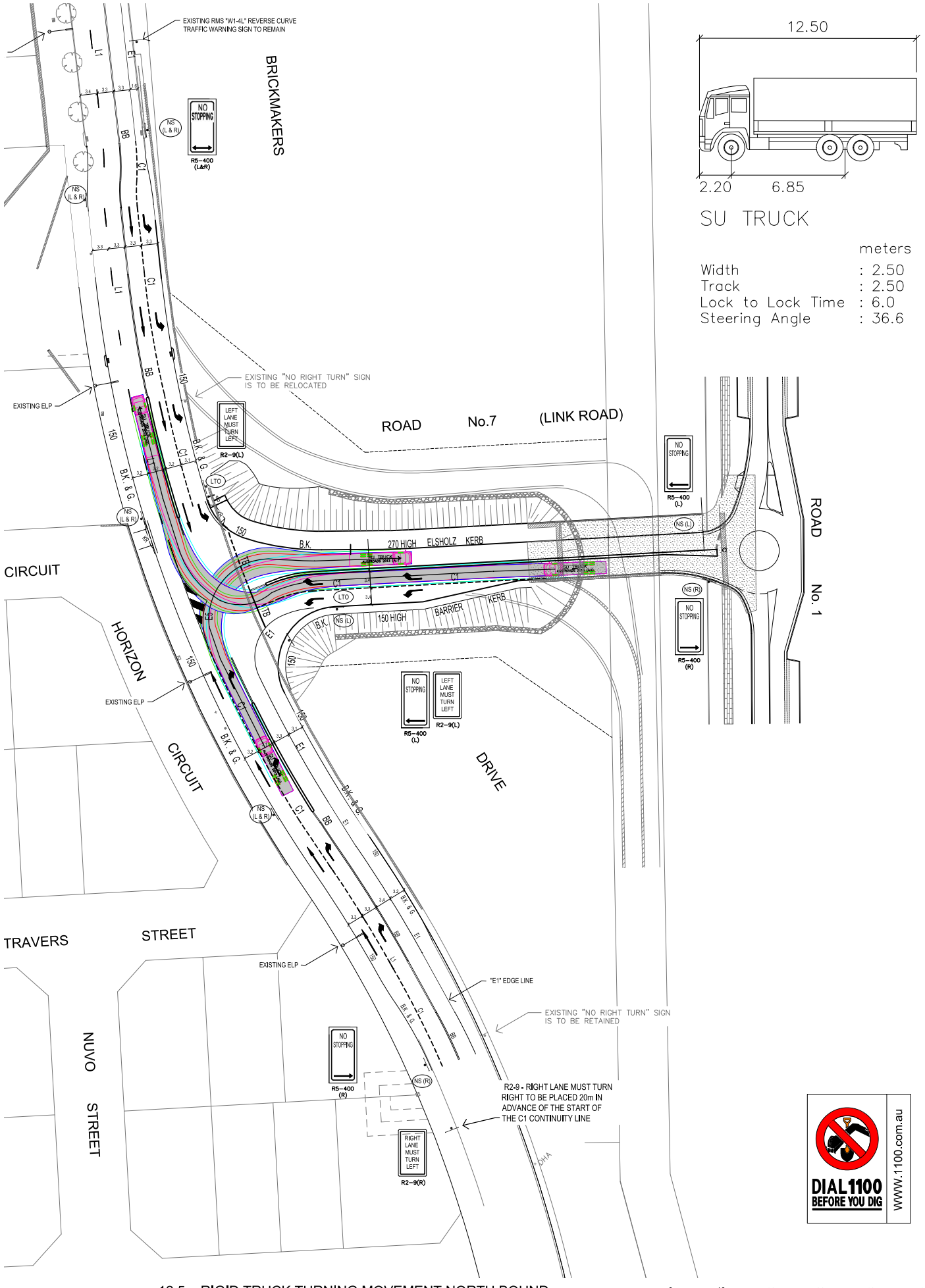
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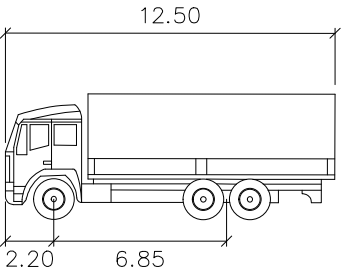
- LEGEND
- FRONT TIRES
 - REAR TIRES
 - FRONT CLEARANCE
 - REAR CLEARANCE
 - VEHICLE BODY



12.5m RIGID TRUCK TURNING MOVEMENT SOUTH BOUND
SCALE 1:500



12.5m RIGID TRUCK TURNING MOVEMENT NORTH BOUND
SCALE 1:500



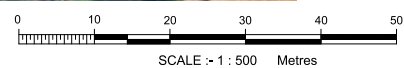
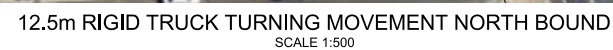
SU TRUCK


Width	: 2.50
Track	: 2.50
Lock to Lock Time	: 6.0
Steering Angle	: 36.6



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A	S.G.	30-05-2019	ISSUED FOR TRAFFIC COMMITTEE APPROVAL	Ratio (A1) : 1:200	Date of Survey : N/A						
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